Aircraft Structures Kirchhoff-Love Plates

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Aircraft Structures - Kirchhoff-Love Plates

Elasticity

- Balance of body *B*
 - Momenta balance
 - Linear
 - Angular
 - Boundary conditions
 - Neumann
 - Dirichlet



• Small deformations with linear elastic, homogeneous & isotropic material

$$- \text{ (Small) Strain tensor } \boldsymbol{\varepsilon} = \frac{1}{2} \left(\boldsymbol{\nabla} \otimes \boldsymbol{u} + \boldsymbol{u} \otimes \boldsymbol{\nabla} \right), \text{ or } \begin{cases} \boldsymbol{\varepsilon}_{ij} = \frac{1}{2} \left(\frac{\partial}{\partial \boldsymbol{x}_i} \boldsymbol{u}_j + \frac{\partial}{\partial \boldsymbol{x}_j} \boldsymbol{u}_i \right) \\ \boldsymbol{\varepsilon}_{ij} = \frac{1}{2} \left(\boldsymbol{u}_{j,i} + \boldsymbol{u}_{i,j} \right) \end{cases}$$

– Hooke's law
$$oldsymbol{\sigma}=\mathcal{H}:oldsymbol{arepsilon}$$
 , or $oldsymbol{\sigma}_{ij}=\mathcal{H}_{ijkl}oldsymbol{arepsilon}_{kl}$

with
$$\mathcal{H}_{ijkl} = \underbrace{\frac{E\nu}{(1+\nu)(1-2\nu)}}_{\lambda=K-2\mu/3} \delta_{ij}\delta_{kl} + \underbrace{\frac{E}{1+\nu}}_{2\mu} \left(\frac{1}{2}\delta_{ik}\delta_{jl} + \frac{1}{2}\delta_{il}\delta_{jk}\right)$$

- Inverse law $\varepsilon = \mathcal{G} : \sigma$ $\lambda = K - 2\mu/3$

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with

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 $\mathcal{G}_{ijkl} = \frac{1+\nu}{E} \left(\frac{1}{2}\delta_{ik}\delta_{jl} + \frac{1}{2}\delta_{il}\delta_{jk}\right) - \frac{\nu}{E}\delta_{ij}\delta_{kl}$



Université de Liège **Reissner-Mindlin equations summary**

- **Deformations (small transformations)** E_3 - In plane membrane • $\varepsilon_{\alpha\beta} = \frac{\boldsymbol{u}_{\alpha,\beta} + \boldsymbol{u}_{\beta,\alpha}}{2}$ Curvature _ • $\kappa_{\alpha\beta} = \frac{\Delta t_{\alpha,\beta} + \Delta t_{\beta,\alpha}}{2}$ А \mathbf{E}_1 - Out-of-plane sliding • $\gamma_lpha = oldsymbol{u}_{3,lpha} + oldsymbol{\Delta} oldsymbol{t}_lpha$ E_3 Z. $\theta_{v_{i}} \Delta t$ х $1/\Delta t_{\mu}^{\mu}$ κΈ. 2013-2014 3 Aircraft Structures - Kirchhoff-Love Plates



 E_2

 $1/\kappa_{11}$

 E_2

Reissner-Mindlin equations summary

- Resultant stresses in linear elasticity
 - Membrane stress
 - $\tilde{n}^{\alpha\beta} = \mathcal{H}_n^{\alpha\beta\gamma\delta} \varepsilon_{\gamma\delta}$
 - Bending stress
 - $\tilde{m}^{\alpha\beta} = \mathcal{H}_m^{\alpha\beta\gamma\delta}\kappa_{\gamma\delta}$
 - Out-of-plane shear stress
 - $\tilde{q}^{\alpha} = \frac{1}{2} \mathcal{H}_{q}^{\alpha\beta} \gamma_{\beta}$





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- Resultant Hooke tensor in linear elasticity
 - Membrane mode • $\mathcal{H}_{n}^{\alpha\beta\gamma\delta} = \frac{h_{0}E}{1-\nu^{2}} \left[\nu\delta^{\alpha\beta}\delta^{\gamma\delta} + \frac{1-\nu}{2} \left(\delta^{\alpha\gamma}\delta^{\beta\delta} + \delta^{\alpha\delta}\delta^{\beta\gamma}\right) \right]$
 - Bending mode

•
$$\mathcal{H}_{m}^{\alpha\beta\gamma\delta} = \frac{h_{0}^{3}E}{12\left(1-\nu^{2}\right)} \left[\nu\delta^{\alpha\beta}\delta^{\gamma\delta} + \frac{1-\nu}{2}\left(\delta^{\alpha\gamma}\delta^{\beta\delta} + \delta^{\alpha\delta}\delta^{\beta\gamma}\right)\right]$$

Shear mode

•
$$\mathcal{H}_q^{\alpha\beta} = \frac{Eh_0}{1+\nu} \frac{A'}{A} \delta^{\alpha\beta}$$





Resultant equations

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– Membrane mode

•
$$(\boldsymbol{n}^{lpha})_{,lpha}+ar{\boldsymbol{n}}=ar{
ho}\ddot{\boldsymbol{u}}$$

$$\begin{array}{ll} \bullet & \boldsymbol{n}^{\alpha} = \tilde{n}^{\alpha\beta} \boldsymbol{E}_{\beta} + \tilde{q}^{\alpha} \boldsymbol{E}_{3} \\ & - & \tilde{n}^{\alpha\beta} = \mathcal{H}_{n}^{\alpha\beta\gamma\delta} \varepsilon_{\gamma\delta} & \quad \text{with} \quad \varepsilon_{\alpha\beta} = \frac{\boldsymbol{u}_{\alpha,\beta} + \boldsymbol{u}_{\beta,\alpha}}{2} \\ & - & \tilde{q}^{\alpha} = \frac{1}{2} \mathcal{H}_{q}^{\alpha\beta} \gamma_{\beta} & \quad \text{with} \quad \gamma_{\alpha} = \boldsymbol{u}_{3,\alpha} + \boldsymbol{\Delta} \boldsymbol{t}_{\alpha} \end{array}$$

• Clearly, the solution can be directly computed in plane Oxy (constant \mathcal{H}_n)



• Remaining equation along E³: $\mathcal{H}_q^{\alpha\beta} \frac{u_{3,\beta\alpha} + \Delta t_{\beta,\alpha}}{2} + \bar{n}_3 = \bar{\rho}\ddot{u}_3$



- Resultant equations (2)
 - Bending mode

•
$$\ddot{\boldsymbol{t}}I_p = \bar{\tilde{\boldsymbol{m}}} - (\boldsymbol{n}^3 - \lambda \boldsymbol{E}_3) + (\tilde{\boldsymbol{m}}^{\alpha})_{,\alpha}$$

$$\begin{array}{l} \tilde{\boldsymbol{m}}^{\alpha} = \tilde{\boldsymbol{m}}^{\alpha\beta} \boldsymbol{E}_{\beta} \,\, \boldsymbol{\&} \,\, \boldsymbol{n}^{3} = \tilde{q}^{\alpha} \boldsymbol{E}_{\alpha} \\ & - \,\tilde{\boldsymbol{m}}^{\alpha\beta} = \mathcal{H}_{m}^{\alpha\beta\gamma\delta} \kappa_{\gamma\delta} \quad \text{with} \,\, \kappa_{\alpha\beta} = \frac{\boldsymbol{\Delta} t_{\alpha,\beta} + \boldsymbol{\Delta} t_{\beta,\alpha}}{2} \\ & - \,\, \tilde{q}^{\alpha} = \frac{1}{2} \mathcal{H}_{q}^{\alpha\beta} \gamma_{\beta} \quad \text{with} \,\, \gamma_{\alpha} = \boldsymbol{u}_{3,\alpha} + \boldsymbol{\Delta} t_{\alpha} \end{array}$$

• Solution is obtained by projecting into the plane Oxy (constant $\mathcal{H}_q, \mathcal{H}_m$)

$$- I_p \ddot{\Delta t}_{\alpha} = \bar{\tilde{m}}_{\alpha} - \frac{1}{2} \mathcal{H}_q^{\alpha\beta} \left(\boldsymbol{u}_{3,\beta} + \Delta t_{\beta} \right) + \mathcal{H}_m^{\alpha\beta\gamma\delta} \frac{\Delta t_{\gamma,\delta\beta} + \Delta t_{\delta,\gamma\beta}}{2}$$

- 2 equations (α =1, 2) with 3 unknowns (Δt_1 , Δt_2 , u_3)

- Use remaining equation
$$\ {\cal H}_q^{lphaeta} {{f u}_{3,etalpha}+\Delta t_{eta,lpha}\over 2}+ar{m n}_3=ar{
ho}\ddot{m u}_3$$





- Resultant equations (3)
 - Bending mode (2)
 - 3 equations with 3 unknowns

- To be completed by BCs
 - Low order constrains
 - » Displacement $\, oldsymbol{u}_3 = ar{oldsymbol{u}}_3 \,$ or
 - » Shearing

$$\mathcal{H}_q^{\alpha\beta} \frac{\boldsymbol{u}_{3,\beta} + \boldsymbol{\Delta} \boldsymbol{t}_\beta}{2} \nu_\alpha = \bar{T}$$

- High order
 - » Rotation $\Delta t = ar{\Delta t}$ or
 - » Bending

$$ilde{m}^{lpha}_{eta}
u_{lpha}=\mathcal{H}^{lphaeta\gamma\delta}_{m}rac{oldsymbol{\Delta} t_{\gamma,\delta}+oldsymbol{\Delta} t_{\delta,\gamma}}{2}
u_{lpha}=ar{M}_{eta}$$









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- Resultant equations (4)
 - Remarks
 - Compare bending equations

$$- I_{p}\ddot{\Delta t}_{\alpha} = \bar{\tilde{m}}_{\alpha} - \frac{1}{2}\mathcal{H}_{q}^{\alpha\beta}\left(\boldsymbol{u}_{3,\beta} + \Delta t_{\beta}\right) + \mathcal{H}_{m}^{\alpha\beta\gamma\delta}\frac{\Delta t_{\gamma,\delta\beta} + \Delta t_{\delta,\gamma\beta}}{2} \\ - \mathcal{H}_{q}^{\alpha\beta}\frac{\boldsymbol{u}_{3,\beta\alpha} + \Delta t_{\beta,\alpha}}{2} + \bar{\boldsymbol{n}}_{3} = \bar{\rho}\ddot{\boldsymbol{u}}_{3}$$

- With Timoshenko beam equations

$$\frac{\partial}{\partial_x} \left(EI \frac{\partial \theta_y}{\partial x} \right) - \mu A' \left(\theta_y + \partial_x \boldsymbol{u}_z \right) = 0$$
$$\frac{\partial}{\partial x} \left(\mu A' \left(\theta_y + \partial_x \boldsymbol{u}_z \right) \right) = -f$$



- Membrane and bending equations are uncoupled
 - No initial curvature
 - Small deformations (equilibrium on non curved configuration)





Shear effect

- Kirchhoff-Love assumption
 - Beam analogy
 - Timoshenko beam equations

$$\frac{\partial}{\partial_x} \left(EI \frac{\partial \theta_y}{\partial x} \right) - \mu A' (\theta_y + \partial_x u_z) = 0$$
$$\frac{\partial}{\partial x} \left(\mu A' (\theta_y + \partial_x u_z) \right) = -f$$

• Euler-Bernoulli equations

$$\frac{\partial^{2}}{\partial x^{2}}\left(EI\frac{\partial^{2}\boldsymbol{u}_{z}}{\partial x^{2}}\right)=f\left(x\right)$$

It has been assumed that

the cross-section remains planar and perpendicular to the neutral axis

$$- u_{z,x} = -\theta_y$$

- Validity:
(*EI*)/(*L*²)1'µ) << 1

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– The same assumption can be made for plates









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Shear effect

• Kirchhoff-Love assumption (2)

- As
•
$$\mathcal{H}_{m}^{\alpha\beta\gamma\delta} = \underbrace{\frac{h_{0}^{3}E}{12(1-\nu^{2})}}_{2} \left[\nu\delta^{\alpha\beta}\delta^{\gamma\delta} + \frac{1-\nu}{2}\left(\delta^{\alpha\gamma}\delta^{\beta\delta} + \delta^{\alpha\delta}\delta^{\beta\gamma}\right)\right]$$

•
$$\mathcal{H}_q^{\alpha\beta} = \frac{Eh_0}{1+\nu} \frac{A'}{A} \delta^{\alpha\beta}$$



Kirchhoff-Love assumption requires

•
$$\frac{D(1+\nu)A}{L^2Eh_0A'} = \frac{h_0^2A}{12L^2(1-\nu)A'} \ll 1$$

• Where *L* is a characteristic distance





Plate kinematics

- Description
 - In the reference frame E_i
 - The plate is defined by



- Mapping of the (deformed) plate
 - Neutral plane $arphi_0\left(\xi^1,\,\xi^2
 ight)=\xi^lpha E_lpha$ lpha =1 or 2, I = 1, 2 or 3
 - Cross section $oldsymbol{t}_0(\xi^1,\,\xi^2)\,,\,\,\|oldsymbol{t}\|=1$ with $oldsymbol{t}_0=oldsymbol{E}_3$
 - Initial plate S_0

- $S_0 = \mathcal{A} \times [-h_0/2 h_0/2]$, for a plate of initial thickness h_0

-
$$\boldsymbol{X} = \boldsymbol{\Phi}_0\left(\xi^I\right) = \boldsymbol{\varphi}_0\left(\xi^{lpha}\right) + \xi^3 \boldsymbol{t}_0(\xi^1,\,\xi^2)$$

• Deformed plate S

-
$$\boldsymbol{x} = \boldsymbol{\Phi}\left(\xi^{I}\right) = \boldsymbol{\varphi}\left(\xi^{lpha}\right) + \xi^{3}\boldsymbol{t}(\xi^{1},\,\xi^{2})$$



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Plate kinematics

• Kirchhoff-Love plate

- Assumptions
 - Kirchhoff (cross section remains plane)

$$oldsymbol{t}_0 = oldsymbol{E}_3 \hspace{0.5cm} oldsymbol{\&} \hspace{0.5cm} oldsymbol{t} = rac{oldsymbol{arphi}_{,1} \wedge oldsymbol{arphi}_{,2}}{\|oldsymbol{arphi}_{,1} \wedge oldsymbol{arphi}_{,2}\|}$$



• Small deformations $\mathcal{S} \simeq \mathcal{S}_0$, $\boldsymbol{\nabla} \simeq \boldsymbol{\nabla}_0$, $\int_{\mathcal{S}} \simeq \int_{\mathcal{S}_0}$

- Displacement field $\varphi = \varphi_0 + u$ with $\|u\| << \sqrt[3]{|\mathcal{S}_0|}$ Kinematics

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Kirchhoff-Love theory

- Deformations (small transformations)
 - Reissner-Mindlin

•
$$\varepsilon_{\alpha\beta} = \frac{u_{\alpha,\beta} + u_{\beta,\alpha}}{2}$$

• $\kappa_{\alpha\beta} = \frac{\Delta t_{\alpha,\beta} + \Delta t_{\beta,\alpha}}{2}$
• $\gamma_{\alpha} = u_{3,\alpha} + \Delta t_{\alpha}$

- Using

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$$egin{array}{lll} \Delta t = - oldsymbol{u}_{3,lpha} oldsymbol{E}_lpha\ \Delta t_{,eta} = - oldsymbol{u}_{3,lphaeta} oldsymbol{E}_lpha \end{array}$$

$$\Longrightarrow \begin{cases} \varepsilon_{\alpha\beta} = \frac{\boldsymbol{u}_{\alpha,\beta} + \boldsymbol{u}_{\beta,\alpha}}{2} \\ \kappa_{\alpha\beta} = -\boldsymbol{u}_{3,\alpha\beta} \\ \gamma_{\alpha} = 0 \end{cases}$$







Kirchhoff-Love theory

- Resultant stresses
 - Reissner-Mindlin

•
$$\tilde{n}^{\alpha\beta} = \mathcal{H}^{\alpha\beta\gamma\delta}_{n}\varepsilon_{\gamma\delta}$$

• $\tilde{m}^{\alpha\beta} = \mathcal{H}^{\alpha\beta\gamma\delta}_{m}\kappa_{\gamma\delta}$
• $\tilde{q}^{\alpha} = \frac{1}{2}\mathcal{H}^{\alpha\beta}_{q}\gamma_{\beta}$

- Using







Resultant equations

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- Membrane mode

•
$$(\boldsymbol{n}^{lpha})_{,lpha}+ar{\boldsymbol{n}}=ar{
ho}\ddot{\boldsymbol{u}}$$

•
$$n^{\alpha} = \tilde{n}^{\alpha\beta} E_{\beta} + \tilde{q}^{\alpha} E_{3}$$

- $\tilde{n}^{\alpha\beta} = \mathcal{H}_{n}^{\alpha\beta\gamma\delta} \varepsilon_{\gamma\delta}$ with $\varepsilon_{\alpha\beta} = \frac{u_{\alpha,\beta} + u_{\beta,\alpha}}{2}$
- $\tilde{q}^{\alpha} = 0$

• Clearly, the solution can be directly computed in plane Oxy (constant \mathcal{H}_n)

$$\begin{array}{l} - \ \mathcal{H}_{n}^{\alpha\beta\gamma\delta} \frac{\boldsymbol{u}_{\gamma,\delta\alpha} + \boldsymbol{u}_{\delta,\gamma\alpha}}{2} + \bar{\boldsymbol{n}}_{\beta} = \bar{\rho}\ddot{\boldsymbol{u}}_{\beta} \\ - \ \text{Boundary conditions} \\ & \text{Neumann } \boldsymbol{u}_{\alpha} = \bar{\boldsymbol{u}}_{\alpha} \\ & \text{Nichlet} \\ & \boldsymbol{n}_{\beta}^{\alpha}\nu_{\alpha} = \mathcal{H}_{n}^{\alpha\beta\gamma\delta} \frac{\boldsymbol{u}_{\gamma,\delta\alpha} + \boldsymbol{u}_{\delta,\gamma\alpha}}{2} \nu_{\alpha} = \bar{\boldsymbol{n}}_{\beta} \\ \end{array}$$



- Resultant equations (2)
 - Bending mode

•
$$\ddot{\boldsymbol{t}}I_p = ar{ extbf{ ilde{m}}} - ig(oldsymbol{n}^3 - \lambda oldsymbol{E}_3 ig) + ig(oldsymbol{ ilde{m}}^lpha ig)_{,lpha}$$

•
$$\tilde{m}^{\alpha} = \tilde{m}^{\alpha\beta} E_{\beta} \& n^{3} = \tilde{q}^{\alpha} E_{\alpha}$$

- $\tilde{m}^{\alpha\beta} = \mathcal{H}_{m}^{\alpha\beta\gamma\delta} \kappa_{\gamma\delta}$ with $\kappa_{\alpha\beta} = -u_{3,\alpha\beta}$
- $\tilde{q}^{\alpha} = 0$

• Solution is obtained by projecting into the plane Oxy (constant \mathcal{H}_m)

$$-I_p \ddot{\boldsymbol{u}}_{3,lpha} = ar{ ilde{\boldsymbol{m}}}_{lpha} - \mathcal{H}_m^{lphaeta\gamma\delta} \boldsymbol{u}_{3,\gamma\deltaeta}$$

- 1 higher-order equation with 1 unknown (u_3)

- With
$$\mathcal{H}_{m}^{\alpha\beta\gamma\delta} = \frac{h_{0}^{3}E}{12\left(1-\nu^{2}\right)} \left[\nu\delta^{\alpha\beta}\delta^{\gamma\delta} + \frac{1-\nu}{2}\left(\delta^{\alpha\gamma}\delta^{\beta\delta} + \delta^{\alpha\delta}\delta^{\beta\gamma}\right)\right]$$

Compare to Euler-Bernoulli beams

$$\frac{\partial^2}{\partial x^2} \left(EI \frac{\partial^2 \boldsymbol{u}_z}{\partial x^2} \right) = f(x)$$

- How to include pressure effect?

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Kirchhoff-Love theory

- Applied pressure
 - 1 higher-order equation with 1 unknown

•
$$-I_p \ddot{\boldsymbol{u}}_{3,lpha} = ar{ ilde{\boldsymbol{m}}}_{lpha} - \mathcal{H}_m^{lphaeta\gamma\delta} \boldsymbol{u}_{3,\gamma\deltaeta}$$

- In case of pressure loading p
 - Assume static problem (and constant \mathcal{H}_m)

•
$$ar{ ilde{m}} = \left(oldsymbol{\sigma} \cdot oldsymbol{E}^3 \xi^3
ight) \Big|_{-rac{h_0}{2}}^{rac{h_0}{2}} + \int_{-rac{h_0}{2}}^{rac{h_0}{2}} \xi^3 oldsymbol{b} d\xi^3$$

has no component along α in case

of pressure loading

- Use energy conservation
 - Work of external forces

$$\delta W_{\text{ext}} = \int_{\mathcal{A}} p \delta \boldsymbol{u}_3 d\mathcal{A} + \int_{\partial_N \mathcal{A}} \bar{T} \delta \boldsymbol{u}_3 - \int_{\partial_M \mathcal{A}} \bar{M} \delta \boldsymbol{u}_{3,\alpha} \nu_\alpha dl$$

• Bending part of internal energy (as problems are uncoupled)?







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- Applied pressure (2)
 - Internal energy
 - Last lecture we considered the decomposition

$$-\varepsilon = \varepsilon_{\alpha\beta} E^{\alpha} \otimes E^{\beta} + \underbrace{\delta_{\alpha}}_{\alpha\beta} (E^{3} \otimes E^{\alpha} + E^{\alpha} \otimes E^{3}) + \xi^{3} \kappa_{\alpha\beta} E^{\alpha} \otimes E^{\beta} + \lambda_{h} E^{3} \otimes E^{3}$$
$$-\sigma^{\alpha\beta} = \sigma_{\tilde{n}}^{\alpha\beta} + \xi^{3} \sigma_{\tilde{m}}^{\alpha\beta} \frac{\gamma_{a}/2}{(1-\nu^{2})} \kappa_{\gamma\gamma} \delta^{\alpha\beta} + \frac{E}{1+\nu} \frac{\kappa_{\alpha\beta} + \kappa_{\beta\alpha}}{2}$$
with $\sigma_{\tilde{m}}^{\alpha\beta} = \frac{E\nu}{(1-\nu^{2})} \kappa_{\gamma\gamma} \delta^{\alpha\beta} + \frac{E}{1+\nu} \frac{\kappa_{\alpha\beta} + \kappa_{\beta\alpha}}{2}$

• Bending contribution to internal energy

$$- \delta E_{\text{int}} = \int_{\mathcal{A}} \int_{-\frac{h_0}{2}}^{\frac{h_0}{2}} (\xi^3)^2 \boldsymbol{\sigma}_{\tilde{m}} : \kappa_{ij} \boldsymbol{E}^I \otimes \boldsymbol{E}^J d\xi^3 d\mathcal{A}$$

$$\implies \delta E_{\text{int}} = \int_{\mathcal{A}} \int_{-\frac{h_0}{2}}^{\frac{h_0}{2}} (\xi^3)^2 d\xi^3 \boldsymbol{\sigma}_{\tilde{m}}^{\alpha\beta} \kappa_{\alpha\beta} d\mathcal{A} = \int_{\mathcal{A}} \frac{h_0^3}{12} \boldsymbol{\sigma}_{\tilde{m}}^{\alpha\beta} \kappa_{\alpha\beta} d\mathcal{A}$$

$$- \text{ As } \tilde{\boldsymbol{m}}^{\alpha} = \frac{h_0^3}{12} \boldsymbol{\sigma}_{\tilde{m}}^{\alpha\beta} \boldsymbol{E}_{\beta} = \tilde{\boldsymbol{m}}^{\alpha\beta} \boldsymbol{E}_{\beta}$$

$$\implies \delta E_{\text{int}} = \int_{\mathcal{A}} \tilde{\boldsymbol{m}}^{\alpha\beta} \delta \kappa_{\alpha\beta} d\mathcal{A}$$

$$- \text{ Remark other contributions (for the uncoupled cases) follow plates}$$

$$\gg \delta E_{\text{int}} = \int_{\mathcal{A}} \tilde{\boldsymbol{m}}^{\alpha\beta} \delta \kappa_{\alpha\beta} d\mathcal{A} + \int_{\mathcal{A}} \tilde{\boldsymbol{n}}^{\alpha\beta} \delta \varepsilon_{\alpha\beta} d\mathcal{A} + \int_{\mathcal{A}} \tilde{\boldsymbol{q}}^{\alpha} \delta \gamma_{\alpha} d\mathcal{A}$$
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- Applied pressure (3)
 - Energy conservation

•
$$\delta E_{\text{int}} = \int_{\mathcal{A}} \tilde{m}^{\alpha\beta} \delta \kappa_{\alpha\beta} d\mathcal{A}$$

• $\delta W_{\text{ext}} = \int_{\mathcal{A}} p \delta \boldsymbol{u}_3 d\mathcal{A} + \int_{\partial_N \mathcal{A}} \bar{T} \delta \boldsymbol{u}_3 - \int_{\partial_M \mathcal{A}} \bar{M} \delta \boldsymbol{u}_{3,\alpha} \nu_{\alpha} d\boldsymbol{u}_3$

• As
$$\kappa_{lphaeta}=-oldsymbol{u}_{3,lphaeta}$$

$$\implies -\int_{\mathcal{A}} \tilde{m}^{\alpha\beta} \delta \boldsymbol{u}_{3,\alpha\beta} d\mathcal{A} = \int_{\mathcal{A}} p \delta \boldsymbol{u}_3 d\mathcal{A} + \int_{\partial_N \mathcal{A}} \bar{T} \delta \boldsymbol{u}_3 dl - \int_{\partial_M \mathcal{A}} \bar{M} \delta \boldsymbol{u}_{3,\alpha} \nu_\alpha dl$$





Applied pressure (4)

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- Resulting bending equations

$$-\int_{\mathcal{A}}\tilde{m}^{\alpha\beta}\delta\boldsymbol{u}_{3,\alpha\beta}d\mathcal{A} = \int_{\mathcal{A}}p\delta\boldsymbol{u}_{3}d\mathcal{A} + \int_{\partial_{N}\mathcal{A}}\bar{T}\delta\boldsymbol{u}_{3}dl - \int_{\partial_{M}\mathcal{A}}\bar{M}\delta\boldsymbol{u}_{3,\alpha}\nu_{\alpha}dl$$

Double integration by parts

$$-\int_{\partial \mathcal{A}} \tilde{m}^{\alpha\beta} \delta \mathbf{u}_{3,\alpha} \nu_{\beta} dl + \int_{\mathcal{A}} \tilde{m}^{\alpha\beta}_{,\beta} \delta \mathbf{u}_{3,\alpha} d\mathcal{A} = \int_{\mathcal{A}} p \delta \mathbf{u}_{3} d\mathcal{A} + \int_{\partial \mathcal{N}\mathcal{A}} \bar{T} \delta \mathbf{u}_{3} dl - \int_{\partial \mathcal{M}\mathcal{A}} \bar{M} \delta \mathbf{u}_{3,\alpha} \nu_{\alpha} dl$$

$$-\int_{\partial \mathcal{A}} \tilde{m}^{\alpha\beta} \delta \mathbf{u}_{3,\alpha} \nu_{\beta} dl + \int_{\partial \mathcal{A}} \tilde{m}^{\alpha\beta}_{,\beta} \delta \mathbf{u}_{3} \nu_{\alpha} dl - \int_{\mathcal{A}} \tilde{m}^{\alpha\beta}_{,\beta\alpha} \delta \mathbf{u}_{3} d\mathcal{A} = \int_{\mathcal{A}} p \delta \mathbf{u}_{3} d\mathcal{A} + \int_{\partial \mathcal{N}\mathcal{A}} \bar{T} \delta \mathbf{u}_{3} dl - \int_{\partial \mathcal{M}\mathcal{A}} \bar{M} \delta \mathbf{u}_{3,\alpha} \nu_{\alpha} dl$$

$$\underbrace{\int_{\mathcal{A}} p \delta \mathbf{u}_{3} d\mathcal{A} + \int_{\partial \mathcal{N}\mathcal{A}} \bar{T} \delta \mathbf{u}_{3} dl - \int_{\partial \mathcal{M}\mathcal{A}} \bar{M} \delta \mathbf{u}_{3,\alpha} \nu_{\alpha} dl}_{\mathbf{n}_{0} = \nu_{\alpha} E^{\alpha}} \underbrace{\int_{\mathbf{n}_{0} = \nu_{\alpha} E^{\alpha}} P \int_{\mathbf{n}_{0} = \nu_{\alpha} E^{\alpha}} P \int_{\mathbf{n}_{0} = \nu_{\alpha} E^{\alpha}} \frac{P \int_{\mathbf{n}_{0} = \nu_{\alpha} E^{\alpha}}{\mathbf{n}_{0} = \nu_{\alpha} E^{\alpha}} \underbrace{f_{\alpha} = v_{\alpha} E^{\alpha}}_{\mathbf{n}_{0} = v_{\alpha} E^{\alpha}}_{\mathbf{n}_{0} = v_{\alpha} E^{\alpha}}_{\mathbf{n}_{0} = v_{\alpha} E^{\alpha}} \underbrace{f_{\alpha} = v_{\alpha} E^{\alpha}}_{\mathbf{n}_{0} = v_{\alpha} E^{\alpha}} \underbrace{f_{\alpha} = v_{\alpha} E^{\alpha}}_{\mathbf{n}_{0} = v_{\alpha}$$

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- Applied pressure (5)
 - Resulting bending equations (2)

$$-\int_{\partial\mathcal{A}}\tilde{m}^{\alpha\beta}\delta\boldsymbol{u}_{3,\alpha}\nu_{\beta}dl + \int_{\partial\mathcal{A}}\tilde{m}^{\alpha\beta}_{,\beta}\delta\boldsymbol{u}_{3}\nu_{\alpha}dl - \int_{\mathcal{A}}\tilde{m}^{\alpha\beta}_{,\beta\alpha}\delta\boldsymbol{u}_{3}d\mathcal{A} = \int_{\mathcal{A}}p\delta\boldsymbol{u}_{3}d\mathcal{A} + \int_{\partial_{N}\mathcal{A}}\bar{T}\delta\boldsymbol{u}_{3}dl - \int_{\partial_{M}\mathcal{A}}\bar{M}\delta\boldsymbol{u}_{3,\alpha}\nu_{\alpha}dl$$

• Using $\tilde{m}^{lphaeta} = \mathcal{H}_m^{lphaeta\gamma\delta}\kappa_{\gamma\delta}$, $\kappa_{lphaeta} = -\boldsymbol{u}_{3,lphaeta}$ and essential boundary conditions

- On
$$\mathcal{A}$$
: $\left(\mathcal{H}_{m}^{\alpha\beta\gamma\delta}\boldsymbol{u}_{3,\gamma\delta}\right)_{,\alpha\beta}=p$

- On
$$\partial_{\scriptscriptstyle N} \mathcal{A}$$
: $- \left(\mathcal{H}_m^{lphaeta\gamma\delta} oldsymbol{u}_{3,\gamma\delta}
ight)_{,eta}
u_lpha = ar{T}$

- On
$$\partial_{M} \mathcal{A}$$
: $- \left(\mathcal{H}_{m}^{\alpha\beta\gamma\delta} \boldsymbol{u}_{3,\gamma\delta} \right) \nu_{\beta} = \bar{M} \nu_{\alpha}$



- Applied pressure (6)
 - Comparison with Euler-Bernoulli beams

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- On
$$\mathcal{A}$$
: $\left(\mathcal{H}_m^{\alpha\beta\gamma\delta}\boldsymbol{u}_{3,\gamma\delta}\right)_{,\alpha\beta}=p$

Beams

0

$$\frac{\partial^2}{\partial x^2} \left(EI \frac{\partial^2 \boldsymbol{u}_z}{\partial x^2} \right) = f\left(x \right)$$

$$- \operatorname{On} \partial_{N} \mathcal{A}: - \left(\mathcal{H}_{m}^{\alpha\beta\gamma\delta}\boldsymbol{u}_{3,\gamma\delta}\right)_{,\beta} \nu_{\alpha} = \bar{T} \qquad - \frac{\partial}{\partial x} \left(EI\frac{\partial^{2}\boldsymbol{u}_{z}}{\partial x^{2}}\right)\Big|_{0,L} = \bar{T}_{z}\Big|_{0,L}$$

- On
$$\partial_{M} \mathcal{A}$$
: $- \left(\mathcal{H}_{m}^{\alpha\beta\gamma\delta} \boldsymbol{u}_{3,\gamma\delta} \right) \nu_{\beta} = \bar{M} \nu_{\alpha}$

$$-EI\frac{\partial^2 \boldsymbol{u}_z}{\partial x^2}\Big|_{0,L} = \bar{M}_{xx}\Big|_{0,L}$$



Kirchhoff-Love example

- Example: simply supported plate - Equations • $(\mathcal{H}_{m}^{\alpha\beta\gamma\delta}u_{3,\gamma\delta})_{,\alpha\beta} = p$ with $\mathcal{H}_{m}^{\alpha\beta\gamma\delta} = \underbrace{\frac{h_{0}^{3}E}{12(1-\nu^{2})}}_{2} \left[\nu\delta^{\alpha\beta}\delta^{\gamma\delta} + \frac{1}{2}\left(\delta^{\alpha\gamma}\delta^{\beta\delta} + \delta^{\alpha\delta}\delta^{\beta\gamma}\right)\right]$
 - Bending couple can be rewritten

$$\mathcal{H}_{m}^{\alpha\beta\gamma\delta}\boldsymbol{u}_{3,\gamma\delta} = D\nu\delta^{\alpha\beta}\left(\boldsymbol{u}_{3,11} + \boldsymbol{u}_{3,22}\right) + D\left(1 - \nu\right)\boldsymbol{u}_{3,\alpha\beta}$$
$$\implies \left(\mathcal{H}_{m}^{\alpha\beta\gamma\delta}\boldsymbol{u}_{3,\gamma\delta}\right)_{,\alpha\beta} = D\nu\left(\partial_{xx} + \partial_{yy}\right)\left(\boldsymbol{u}_{3,11} + \boldsymbol{u}_{3,22}\right) + D\left(1 - \nu\right)\left(\boldsymbol{u}_{3,1111} + 2\boldsymbol{u}_{3,1122} + \boldsymbol{u}_{3,2222}\right)$$

• General equation: $D(u_{3,1111} + 2u_{3,1122} + u_{3,2222}) = p(x, y)$

- BCs for
$$x = 0$$
 & $x = a$

• $u_3 = 0$

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• No torque
$$\implies \tilde{m}^{11} = -D(u_{3,11} + \nu u_{3,22})$$

- As on these edges $\boldsymbol{u}_{3,22}=0 \implies \boldsymbol{u}_{3,11}=0$



Kirchhoff-Love example

- Simply supported plate: Methodology
 - Using BCs for x = 0, a & for y = 0, b
 - $u_3 = 0$ $u_3 = 0$
 - $u_{3,11} = 0$ $u_{3,22} = 0$
 - Solution can be represented
 - as a Fourier series

$$\boldsymbol{u}_3 = \sum_{m=1}^{\infty} \sum_{n=1}^{\infty} A_{mn} \sin \frac{m\pi x}{a} \sin \frac{n\pi y}{b}$$



- Coefficients A_{mn} such that $D(u_{3,1111} + 2u_{3,1122} + u_{3,2222}) = p(x, y)$
- Solution for
 - *p*(*x*, *y*) ?
 - $p(x,y) = p_0$?





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- Simply supported plate: General solution ۲
 - Applied pressure can also be expended as a Fourier series

•
$$p(x, y) = \sum_{m=1}^{\infty} \sum_{n=1}^{\infty} a_{mn} \sin \frac{m\pi x}{a} \sin \frac{n\pi y}{b}$$

Coefficient a_{nn} can be evaluated using •

$$-\int_0^a \sin \frac{m\pi x}{a} \sin \frac{m'\pi x}{a} dx = \frac{a}{2} \delta_{mm'}$$

$$-\int_0^b \sin\frac{n\pi x}{b} \sin\frac{n'\pi x}{b} dx = \frac{b}{2}\delta_{nn'}$$

$$E_{1}$$

$$-\int_{0}^{r} \sin \frac{n\pi x}{b} \sin \frac{n\pi x}{b} dx = \frac{b}{2} \delta_{nn'}$$

$$\Longrightarrow \int_{0}^{a} \int_{0}^{b} p(x, y) \sin \frac{m'\pi x}{a} \sin \frac{n'\pi y}{b} dx dy =$$

$$\sum_{m=1}^{\infty} \sum_{n=1}^{\infty} \int_{0}^{a} \int_{0}^{b} a_{mn} \sin \frac{m\pi x}{a} \sin \frac{n\pi y}{b} \sin \frac{m'\pi x}{a} \sin \frac{n'\pi y}{b} dx dy = \frac{ab}{4} a_{m'n'}$$

$$\Longrightarrow a_{mn} = \frac{4}{ab} \int_{0}^{a} \int_{0}^{b} p(x, y) \sin \frac{m\pi x}{a} \sin \frac{n\pi y}{b} dx dy$$





Kirchhoff-Love example

- Simply supported plate: General solution (2)
 - Equations

$$\begin{pmatrix}
p(x, y) = \sum_{m=1}^{\infty} \sum_{n=1}^{\infty} a_{mn} \sin \frac{m\pi x}{a} \sin \frac{n\pi y}{b} \\
u_3 = \sum_{m=1}^{\infty} \sum_{n=1}^{\infty} A_{mn} \sin \frac{m\pi x}{a} \sin \frac{n\pi y}{b} \\
D(u_{3,1111} + 2u_{3,1122} + u_{3,2222}) = p(x, y)
\end{cases}$$



- What remains to be defined are the A_{mn} (as a_{mn} are known)
 - Balance equation yields

$$\sum_{m=1}^{\infty} \sum_{n=1}^{\infty} \left\{ A_{mn} \left[\left(\frac{m\pi}{a} \right)^4 + 2 \left(\frac{m\pi}{a} \right)^2 \left(\frac{n\pi}{b} \right)^2 + \left(\frac{n\pi}{b} \right)^4 \right] - \frac{a_{mn}}{D} \right\}$$
$$\sin \frac{m\pi x}{a} \sin \frac{n\pi y}{b} = 0$$

• Should remain true for any *x*, *y*

$$\implies A_{mn} = \frac{1}{\pi^4 D} \frac{a_{mn}}{\left(\frac{m^2}{a^2} + \frac{n^2}{b^2}\right)^2}$$

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Kirchhoff-Love example

- Simply supported plate: Constant pressure p_0
 - Coefficients a_{mn} • $a_{mn} = \frac{4}{ab} \int_{0}^{a} \int_{0}^{b} p(x, y) \sin \frac{m\pi x}{a} \sin \frac{n\pi y}{b} dx dy$ E_{1} $\implies a_{mn} = \frac{4p_0}{ab} \int_0^a \sin \frac{m\pi x}{a} dx \int_0^b \sin \frac{n\pi y}{b} dy$ - For *m* or *n* even: $a_{mn} = 0$

- For *m* & *n* odd:
$$a_{mn} = \frac{16p_0}{\pi^2 m n}$$

Coefficients A_{mn} for m & n odd:

•
$$A_{mn} = \frac{1}{\pi^4 D} \frac{a_{mn}}{\left(\frac{m^2}{a^2} + \frac{n^2}{b^2}\right)^2} = \frac{16p_0}{D\pi^6 mn \left(\frac{m^2}{a^2} + \frac{n^2}{b^2}\right)^2}$$

Solution of the problem

•
$$\boldsymbol{u}_3 = \sum_{m=1,3,5}^{\infty} \sum_{n=1,3,5}^{\infty} \frac{16p_0}{D\pi^6 mn \left(\frac{m^2}{a^2} + \frac{n^2}{b^2}\right)^2} \sin \frac{m\pi x}{a} \sin \frac{n\pi y}{b}$$





 E_3

 p_0

h

- Simply supported plate: Constant pressure p_0 (2)
 - Solution of the problem

$$\boldsymbol{u}_{3} = \sum_{m=1,3,5}^{\infty} \sum_{n=1,3,5}^{\infty} \frac{16p_{0}}{D\pi^{6}mn\left(\frac{m^{2}}{a^{2}} + \frac{n^{2}}{b^{2}}\right)^{2}} \sin\frac{m\pi x}{a} \sin\frac{n\pi y}{b}$$

- For a square plate

$$\boldsymbol{u}_{3} = \sum_{m=1,3,5}^{\infty} \sum_{n=1,3,5}^{\infty} \frac{16p_{0}a^{4}}{D\pi^{6}mn\left(m^{2}+n^{2}\right)^{2}} \sin\frac{m\pi x}{a} \sin\frac{n\pi y}{a}$$

$A_{mn}D$	m=1	<i>m=3</i>	<i>m</i> =5	<i>m</i> =7
$p_0 a^4$				
<i>n</i> =1	4.161 10 ⁻³	0.055 10 ⁻³	0.005 10 ⁻³	0.001 10 ⁻³
<i>n=3</i>	Sym.	0.006 10 ⁻³	0.001 10 ⁻³	0.002 10 ⁻⁴
<i>n</i> =5	Sym.	Sym.	0.003 10 ⁻⁴	0.001 10 ⁻⁴
<i>n</i> =7	Sym.	Sym.	Sym.	0.004 10 ⁻⁵

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a

 E_{3}

А

 \bigcirc

b

 p_0

- Simply supported plate: Constant pressure p_0 (3)
 - For a square plate (2)

•
$$\boldsymbol{u}_3 = \sum_{m=1,3,5}^{\infty} \sum_{n=1,3,5}^{\infty} \frac{16p_0 a^4}{D\pi^6 mn \left(m^2 + n^2\right)^2} \sin \frac{m\pi x}{a} \sin \frac{n\pi y}{a}$$

• Maximum deflection at *x*=*y*=*a*/2

$$u_{3}\left(\frac{a}{2}, \frac{a}{2}\right) = \frac{p_{0}a^{4}}{D} \sum_{m=1,3,5}^{\infty} \sum_{n=1,3,5}^{\infty} \frac{16}{\pi^{6}mn\left(m^{2}+n^{2}\right)^{2}} \sin\frac{m\pi}{2} \sin\frac{n\pi}{2}$$
$$\simeq 0.0042 \frac{p_{0}a^{4}}{D} \times 10^{-3}$$



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Aircraft Structures - Kirchhoff-Love Plates

- Simply supported plate: Constant pressure p_0 (4)
 - Resultant stress couple

• From
$$u_3 = \sum_{m=1,3,5}^{\infty} \sum_{n=1,3,5}^{\infty} \frac{16p_0}{D\pi^6 mn \left(\frac{m^2}{a^2} + \frac{n^2}{b^2}\right)^2} \sin \frac{m\pi x}{a} \sin \frac{n\pi y}{b}$$

&

• Bending
$$\tilde{m}^{\alpha\beta} = -\mathcal{H}_{m}^{\alpha\beta\gamma\delta} \boldsymbol{u}_{3,\gamma\delta} = -D\left[\nu\delta^{\alpha\beta}\left(\boldsymbol{u}_{3,11} + \boldsymbol{u}_{3,22}\right) + (1-\nu)\,\boldsymbol{u}_{3,\alpha\beta}\right]$$

 $- \tilde{m}^{11} = -D\left(\boldsymbol{u}_{3,11} + \nu\boldsymbol{u}_{3,22}\right)$
 $\implies \tilde{m}^{11} = \frac{16p_0}{\pi^4} \sum_{m=1,3,5}^{\infty} \sum_{n=1,3,5}^{\infty} \frac{\frac{m^2}{a^2} + \nu\frac{n^2}{b^2}}{mn\left(\frac{m^2}{a^2} + \frac{n^2}{b^2}\right)^2} \sin\frac{m\pi x}{a} \sin\frac{n\pi y}{b}$
 $- \tilde{m}^{22} = -D\left(\boldsymbol{u}_{3,22} + \nu\boldsymbol{u}_{3,11}\right)$
 $\implies \tilde{m}^{22} = \frac{16p_0}{\pi^4} \sum_{m=1,3,5}^{\infty} \sum_{n=1,3,5}^{\infty} \frac{\nu\frac{m^2}{a^2} + \frac{n^2}{b^2}}{mn\left(\frac{m^2}{a^2} + \frac{n^2}{b^2}\right)^2} \sin\frac{m\pi x}{a} \sin\frac{n\pi y}{b}$

• Twisting

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$$- \tilde{m}^{12} = -D(1-\nu) u_{3,12}$$

$$\implies \tilde{m}^{12} = -\frac{16p_0(1-\nu)}{\pi^4} \sum_{m=1,3,5}^{\infty} \sum_{n=1,3,5}^{\infty} \frac{1}{ab \left(\frac{m^2}{a^2} + \frac{n^2}{b^2}\right)^2} \cos \frac{m\pi x}{a} \cos \frac{n\pi y}{b}$$
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- Simply supported plate: Constant pressure p_0 (5)
 - Resultant stress couple (2)
 - For a square plate and v=0.3







- Simply supported plate: Constant pressure p_0 (6)
 - Stresses

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• Using

$$\tilde{m}^{i\alpha} = \tilde{m}^{\alpha}_i = \int_{-\frac{h_0}{2}}^{\frac{h_0}{2}} \sigma_{i\alpha} \xi^3 d\xi^3$$

• Direct stresses due to bending

$$- \boldsymbol{\sigma}_{11} = \frac{12\tilde{m}^{11}\xi^3}{h_0^3}$$
$$- \boldsymbol{\sigma}_{22} = \frac{12\tilde{m}^{22}\xi^3}{h_0^3}$$

- Shear stress due to torsion $12 \tilde{m}^{12} \xi^3$

$$- \sigma_{12} = - \frac{1}{h_0^3}$$

- For a square plate (v=0.3)

$$\boldsymbol{\sigma}_{11}\left(\frac{a}{2}, \frac{a}{2}, \frac{h_0}{2}\right) = \boldsymbol{\sigma}_{22}\left(\frac{a}{2}, \frac{a}{2}, \frac{h_0}{2}\right) = \frac{6\tilde{m}^{11}}{h_0^2} = 0.287\frac{p_0a^2}{h_0^2}$$





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- Uncoupled theory of Kirchhoff-Love plates
 - We found for tension • $\mathcal{H}_{n}^{\alpha\beta\gamma\delta} \frac{\boldsymbol{u}_{\gamma,\delta\alpha} + \boldsymbol{u}_{\delta,\gamma\alpha}}{2} + \bar{\boldsymbol{n}}_{\beta} = \bar{\rho}\ddot{\boldsymbol{u}}_{\beta}$ • With $\mathcal{H}_{n}^{\alpha\beta\gamma\delta} = \frac{h_{0}E}{1 - \nu^{2}} \left[\nu\delta^{\alpha\beta}\delta^{\gamma\delta} + \frac{1 - \nu}{2} \left(\delta^{\alpha\gamma}\delta^{\beta\delta} + \delta^{\alpha\delta}\delta^{\beta\gamma}\right)\right]$
 - Completed by appropriate BCs
 - We found in bending

•
$$\left(\mathcal{H}_{m}^{\alpha\beta\gamma\delta}\boldsymbol{u}_{3,\gamma\delta}\right)_{,\alpha\beta}=p$$

• With
$$\mathcal{H}_{m}^{\alpha\beta\gamma\delta} = \frac{h_{0}^{3}E}{12(1-\nu^{2})} \left[\nu\delta^{\alpha\beta}\delta^{\gamma\delta} + \frac{1-\nu}{2}\left(\delta^{\alpha\gamma}\delta^{\beta\delta} + \delta^{\alpha\delta}\delta^{\beta\gamma}\right)\right]$$

 E_{3} $\partial_N \mathcal{A}$ А n $\partial_D \mathcal{A}$ E_2 E. $\widehat{\boldsymbol{n}}_{0} = \boldsymbol{v}_{\alpha} \boldsymbol{E}^{\alpha}$ E_3 p $\partial_N \mathcal{I}$ А $\partial_D A$ E_2 \mathbf{E}_{1} $\widehat{\boldsymbol{n}}_{0} = v_{\alpha} E^{\alpha}$ E_3 $\partial_M \mathcal{A}$ А M $\partial_T \mathcal{A}$

₽₁

 E_2

 $\widehat{\boldsymbol{n}}_0 = \boldsymbol{v}_{\alpha} E^{\alpha}$

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- Completed by appropriate BCs
 - Low order
 - High order

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Aircraft Structures - Kirchhoff-Love Plates

- Uncoupled theory of Kirchhoff-Love plates (2)
 - Uncoupled theory

•
$$\mathcal{H}_{n}^{\alpha\beta\gamma\delta} \frac{\boldsymbol{u}_{\gamma,\delta\alpha} + \boldsymbol{u}_{\delta,\gamma\alpha}}{2} + \bar{\boldsymbol{n}}_{\beta} = \bar{\rho}\ddot{\boldsymbol{u}}_{\beta}$$

• $\left(\mathcal{H}_{m}^{\alpha\beta\gamma\delta}\boldsymbol{u}_{3,\gamma\delta}\right)_{,\alpha\beta} = p$

- Results from the fact that
 - Equilibrium is written on the initial configuration
 - Small deformations assumption
 - This initial configuration is planar
- As equilibrium should be written
 - In deformed configuration, which has • a curvature
 - Tension will affect bending •
 - Second order theory ٠







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- Second order theory: Bending
 - Tension and shearing have a resultant loading in the *z* direction
 - Due to *x* tension

$$(\tilde{n}^{11} + \partial_x \tilde{n}^{11} \delta x) \, \delta y \sin \left(\boldsymbol{u}_{3,1} + \boldsymbol{u}_{3,11} \delta x \right) - \\ \tilde{n}^{11} \delta y \sin \boldsymbol{u}_{3,1}$$

• Second order theory: $sin(x) \implies x$

$$\begin{pmatrix} \tilde{n}^{11} + \partial_x \tilde{n}^{11} \delta x \end{pmatrix} \delta y \boldsymbol{u}_{3,1} + \\ \begin{pmatrix} \tilde{n}^{11} + \partial_x \tilde{n}^{11} \delta x \end{pmatrix} \delta y \boldsymbol{u}_{3,11} \delta x - \tilde{n}^{11} \delta y \boldsymbol{u}_{3,1}$$

• After simplification (and removing δ^3 terms)

$$\left(\tilde{n}^{11}\boldsymbol{u}_{3,11} + \partial_x \tilde{n}^{11}\boldsymbol{u}_{3,1}\right)\delta x \delta y$$

- Due to *y* tension
 - Similarly

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$$\left(\tilde{n}^{22}\boldsymbol{u}_{3,22} + \partial_y \tilde{n}^{22}\boldsymbol{u}_{3,2}\right)\delta x \delta y$$

– Due to yx shearing?





- Second order theory: Bending (2)
 - Tension and shearing have a resultant loading in the z direction (2)
 - Due to *yx* shearing
 - $(\tilde{n}^{21} + \partial_y \tilde{n}^{21} \delta y) \delta x \sin(\boldsymbol{u}_{3,1} + \boldsymbol{u}_{3,12} \delta y) \tilde{n}^{21} \delta x \sin \boldsymbol{u}_{3,1}$
 - Second order theory: $sin(x) \implies x$

$$\begin{array}{l} \left(\tilde{n}^{21} + \partial_y \tilde{n}^{21} \delta y \right) \delta x \boldsymbol{u}_{3,1} + \\ \left(\tilde{n}^{21} + \partial_y \tilde{n}^{21} \delta y \right) \delta x \boldsymbol{u}_{3,12} \delta y - \tilde{n}^{21} \delta x \boldsymbol{u}_{3,1} \end{array}$$

• After simplification (and removing δ^3 terms)

$$\left(\partial_y \tilde{n}^{21} \boldsymbol{u}_{3,1} + \tilde{n}^{21} \boldsymbol{u}_{3,12}\right) \delta x \delta y$$

- Due to *xy* shearing
 - Similarly

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$$\left(\partial_x \tilde{n}^{12} \boldsymbol{u}_{3,2} + \tilde{n}^{12} \boldsymbol{u}_{3,21}\right) \delta x \delta y$$



- Second order theory: Bending (3)
 - Tension and shearing have a resultant loading in the z direction
 - Value on surface $\delta x \delta y$:

$$\left(\tilde{n}^{11} \boldsymbol{u}_{3,11} + \partial_x \tilde{n}^{11} \boldsymbol{u}_{3,1} + \tilde{n}^{22} \boldsymbol{u}_{3,22} + \partial_y \tilde{n}^{22} \boldsymbol{u}_{3,2} + \\ \partial_y \tilde{n}^{21} \boldsymbol{u}_{3,1} + \tilde{n}^{21} \boldsymbol{u}_{3,12} + \partial_x \tilde{n}^{12} \boldsymbol{u}_{3,2} + \tilde{n}^{12} \boldsymbol{u}_{3,21} \right) \delta x \delta y$$

• But tension equilibrium reads

$$\begin{cases} \partial_x \tilde{n}^{11} + \partial_y \tilde{n}^{12} = 0\\ \partial_x \tilde{n}^{12} + \partial_y \tilde{n}^{22} = 0 \end{cases}$$

• Final expression

$$(\tilde{n}^{11}\boldsymbol{u}_{3,11} + \tilde{n}^{22}\boldsymbol{u}_{3,22} + \tilde{n}^{21}\boldsymbol{u}_{3,12} + \tilde{n}^{12}\boldsymbol{u}_{3,21}) \,\delta x \delta y \\ \Longrightarrow \tilde{n}^{\alpha\beta}\boldsymbol{u}_{3,\alpha\beta} \delta x \delta y$$

Using tension equilibrium twice more

$$\tilde{n}^{\alpha\beta}\boldsymbol{u}_{3,\alpha\beta}\delta x\delta y = \left[\left(\tilde{n}^{\alpha\beta}\boldsymbol{u}_{3,\beta} \right)_{,\alpha} - \tilde{n}^{\alpha\beta}_{,\alpha}\boldsymbol{u}_{3,\beta} \right] \delta x\delta y = \left(\tilde{n}^{\alpha\beta}\boldsymbol{u}_{3,\beta} \right)_{,\alpha} \delta x\delta y$$
$$\implies \tilde{n}^{\alpha\beta}\boldsymbol{u}_{3,\alpha\beta}\delta x\delta y = \left[\left(\tilde{n}^{\alpha\beta}\boldsymbol{u}_{3} \right)_{,\alpha\beta} - \left(\tilde{n}^{\alpha\beta}_{,\beta}\boldsymbol{u}_{3} \right)_{,\alpha} \right] \delta x\delta y = \left(\tilde{n}^{\alpha\beta}\boldsymbol{u}_{3} \right)_{,\alpha\beta} \delta x\delta y$$





- Second order theory: Bending (4)
 - First order theory

•
$$\left(\mathcal{H}_{m}^{\alpha\beta\gamma\delta}\boldsymbol{u}_{3,\gamma\delta}\right)_{,\alpha\beta} = p$$

- Second order theory
 - Tension & shearing lead to a force $(\tilde{n}^{\alpha\beta}u_3)_{,\alpha\beta}\delta x\delta y$ along z on a surface $\delta x\delta y$
 - This corresponds to a pressure

$$\left(\tilde{n}^{lphaeta}\boldsymbol{u}_3
ight)_{,lphaeta}$$

New resulting equation

$$(\mathcal{H}_{m}^{\alpha\beta\gamma\delta}\boldsymbol{u}_{3,\gamma\delta})_{,\alpha\beta} = p + (\tilde{n}^{\alpha\beta}\boldsymbol{u}_{3})_{,\alpha\beta}$$
$$\Longrightarrow (\mathcal{H}_{m}^{\alpha\beta\gamma\delta}\boldsymbol{u}_{3,\gamma\delta} - \tilde{n}^{\alpha\beta}\boldsymbol{u}_{3})_{,\alpha\beta} = p$$









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- Second order theory: Energy
 - Equation

$$\left(\mathcal{H}_{m}^{\alpha\beta\gamma\delta}\boldsymbol{u}_{3,\gamma\delta}-\tilde{n}^{\alpha\beta}\boldsymbol{u}_{3}\right)_{,\alpha\beta}=p$$

- Bending energy

• We saw that
$$\delta E_{\text{int}} = \int_{\mathcal{A}} \tilde{m}^{\alpha\beta} \delta \kappa_{\alpha\beta} d\mathcal{A}$$
 with $\begin{cases} \kappa_{\alpha\beta} = -\boldsymbol{u}_{3,\alpha\beta} \\ \tilde{m}^{\alpha\beta} = \mathcal{H}_m^{\alpha\beta\gamma\delta} \kappa_{\gamma\delta} \end{cases}$

• As
$$\tilde{m}^{\alpha\beta} = -\mathcal{H}_{m}^{\alpha\beta\gamma\delta} \boldsymbol{u}_{3,\gamma\delta} = -D\left[\nu\delta^{\alpha\beta}\left(\boldsymbol{u}_{3,11} + \boldsymbol{u}_{3,22}\right) + (1-\nu)\boldsymbol{u}_{3,\alpha\beta}\right]$$

$$\implies E_{\text{int, bending}} = \int_{\mathcal{A}} \frac{D}{2} \left[\nu \left(\boldsymbol{u}_{3,11} + \boldsymbol{u}_{3,22} \right)^2 + (1 - \nu) \left(\boldsymbol{u}_{3,11}^2 + 2\boldsymbol{u}_{3,12}^2 + \boldsymbol{u}_{3,22}^2 \right) \right] d\mathcal{A}$$
$$\implies E_{\text{int, bending}} = \int_{\mathcal{A}} \frac{D}{2} \left[\boldsymbol{u}_{3,11}^2 + \boldsymbol{u}_{3,22}^2 + 2 \left(1 - \nu \right) \boldsymbol{u}_{3,12}^2 + 2\nu \boldsymbol{u}_{3,11} \boldsymbol{u}_{3,22} \right] d\mathcal{A}$$

But part of the bending results from tension





- Second order theory: Energy (2)
 - Bending energy (2)
 - Part of the bending resulting from tension
 - This corresponds to a pressure $(\tilde{n}^{lphaeta}oldsymbol{u}_3)_{,lphaeta}$



$$\implies \delta W_{\text{ext, traction}} = \int_{\mathcal{A}} \tilde{n}^{\alpha\beta} \boldsymbol{u}_{3,\alpha\beta} \, \delta \boldsymbol{u}_3 d\mathcal{A} = \int_{\partial \mathcal{A}} \tilde{n}^{\alpha\beta} \boldsymbol{u}_{3,\alpha} \, \delta \boldsymbol{u}_3 \nu_\beta dl - \int_{\mathcal{A}} \tilde{n}^{\alpha\beta} \boldsymbol{u}_{3,\alpha} \, \delta \boldsymbol{u}_{3,\beta} d\mathcal{A}$$
$$- \operatorname{\mathsf{As}} \left\{ \begin{array}{l} \partial_x \tilde{n}^{11} + \partial_y \tilde{n}^{12} = 0 \\ \partial_x \tilde{n}^{12} + \partial_y \tilde{n}^{22} = 0 \end{array} \right.$$

• Assuming BCs are such that there is no contribution to energy $(u_3=0 \text{ or } u_{3,\alpha}=0)$, and if \tilde{n} is constant with u_3 ,

$$E_{\text{int, traction}} = \frac{1}{2} \int_{\mathcal{A}} \left[\tilde{n}^{11} \boldsymbol{u}_{3,1}^{2} + \tilde{n}^{22} \boldsymbol{u}_{3,2}^{2} + 2\tilde{n}^{12} \boldsymbol{u}_{3,1} \boldsymbol{u}_{3,2} \right] d\mathcal{A}$$

• Total bending energy

$$\implies E_{\text{int}} = \int_{\mathcal{A}} \frac{D}{2} \left[\boldsymbol{u}_{3,11}^2 + \boldsymbol{u}_{3,22}^2 + 2(1-\nu) \, \boldsymbol{u}_{3,12}^2 + 2\nu \boldsymbol{u}_{3,11} \boldsymbol{u}_{3,22} \right] d\mathcal{A} + \frac{1}{2} \int_{\mathcal{A}} \left[\tilde{n}^{11} \boldsymbol{u}_{3,1}^2 + \tilde{n}^{22} \boldsymbol{u}_{3,2}^2 + 2\tilde{n}^{12} \boldsymbol{u}_{3,1} \boldsymbol{u}_{3,2} \right] d\mathcal{A}$$



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- Example: simply supported plate with tension
 - BCs for x = 0, a & for y = 0, b
 - $u_3 = 0$ $u_3 = 0$
 - $u_{3,11} = 0$ $u_{3,22} = 0$ (no torque)
 - Constant tension along Ox
 - Equations
 - $\left(\mathcal{H}_{m}^{\alpha\beta\gamma\delta}\boldsymbol{u}_{3,\gamma\delta}-\tilde{n}^{\alpha\beta}\boldsymbol{u}_{3}\right)_{,\alpha\beta}=p$
 - Can be developed using

$$- \tilde{m}^{\alpha\beta} = -\mathcal{H}_{m}^{\alpha\beta\gamma\delta}\boldsymbol{u}_{3,\gamma\delta} = -D\left[\nu\delta^{\alpha\beta}\left(\boldsymbol{u}_{3,11} + \boldsymbol{u}_{3,22}\right) + (1-\nu)\boldsymbol{u}_{3,\alpha\beta}\right]$$

with $D = \frac{h_{0}^{3}E}{12\left(1-\nu^{2}\right)}$
 $-\left(\tilde{n}^{\alpha\beta}\boldsymbol{u}_{3}\right)_{,\alpha\beta} = \tilde{n}^{11}\boldsymbol{u}_{3,11}$

• Final equation

-
$$D(\boldsymbol{u}_{3,1111} + 2\boldsymbol{u}_{3,1122} + \boldsymbol{u}_{3,2222}) - \tilde{n}^{11}\boldsymbol{u}_{3,11} = p$$

Solution for

• $p(x,y) = p_0$?

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 E_3

 E_1

 \tilde{n}^{11}

 \tilde{n}^{11}

- Simply supported plate with tension & constant pressure p₀
 - Pressure can be expended in a Fourier series

• $p(x, y) = \sum \sum a_{mn} \sin \frac{m\pi x}{2} \sin \frac{n\pi y}{k}$

• With
$$a_{mn} = \frac{4}{ab} \int_0^a \int_0^b p(x, y) \sin \frac{m\pi x}{a} \sin \frac{n\pi y}{b} dx dy$$

- Coefficients a_{mn} in case of constant pressure
 - For *m* or *n* even: $a_{mn} = 0$

• For *m* & *n* odd:
$$a_{mn} = \frac{16p_0}{\pi^2 mn}$$

$$\implies p(x, y) = \frac{16p_0}{\pi^2} \sum_{m=1, 3, 5}^{\infty} \sum_{n=1, 3, 5}^{\infty} \frac{1}{mn} \sin \frac{m\pi x}{a} \sin \frac{n\pi y}{b}$$

Final equation to be solved

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$$\boldsymbol{u}_{3,1111} + 2\boldsymbol{u}_{3,1122} + \boldsymbol{u}_{3,2222} - \frac{\tilde{n}^{11}}{D}\boldsymbol{u}_{3,11} = \frac{16p_0}{D\pi^2} \sum_{m=1,3,5}^{\infty} \sum_{n=1,3,5}^{\infty} \frac{1}{mn} \sin\frac{m\pi x}{a} \sin\frac{n\pi y}{b}$$

 \tilde{n}^{11}



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 \tilde{n}^{11}

 E_{2}

- Simply supported plate with tension & constant pressure p_0 (2)
 - Governing equation to be solved

$$\boldsymbol{u}_{3,1111} + 2\boldsymbol{u}_{3,1122} + \boldsymbol{u}_{3,2222} - \frac{\tilde{n}^{11}}{D} \boldsymbol{u}_{3,11} = \frac{16p_0}{D\pi^2} \sum_{m=1,3,5}^{\infty} \sum_{n=1,3,5}^{\infty} \frac{1}{mn} \sin \frac{m\pi x}{a} \sin \frac{n\pi y}{b}$$

- Considering BCs
 - for x = 0, a & for y = 0, b $\begin{cases} u_3 = 0 \\ u_{3,11} = 0 \end{cases}$ $\begin{cases} u_3 = 0 \\ u_{3,22} = 0 \end{cases}$

The solution can take the form

• $\boldsymbol{u}_3 = \sum_{m=1}^{\infty} \sum_{n=1}^{\infty} A_{mn} \sin \frac{m\pi x}{a} \sin \frac{n\pi y}{b}$



• The governing equation leads to a system of equations with A_{mn} as unknowns

$$\implies \sum_{m=1}^{\infty} \sum_{n=1}^{\infty} A_{mn} \left[\left(\frac{m^4 \pi^4}{a^4} + 2 \frac{m^2 n^2 \pi^4}{a^2 b^2} + \frac{n^4 \pi^4}{b^4} + \frac{\tilde{n}^{11} m^2 \pi^2}{D a^2} \right) \sin \frac{m \pi x}{a} \sin \frac{n \pi y}{b} \right] = \frac{16p_0}{D \pi^2} \sum_{m=1,3,5}^{\infty} \sum_{n=1,3,5}^{\infty} \frac{1}{mn} \sin \frac{m \pi x}{a} \sin \frac{n \pi y}{b}$$





- Simply supported plate with tension & constant pressure p_0 (3)
 - System of equations

•
$$\sum_{m=1}^{\infty} \sum_{n=1}^{\infty} A_{mn} \left[\left(\frac{m^4 \pi^4}{a^4} + 2 \frac{m^2 n^2 \pi^4}{a^2 b^2} + \frac{n^4 \pi^4}{b^4} + \frac{\tilde{n}^{11} m^2 \pi^2}{D a^2} \right) \sin \frac{m \pi x}{a} \sin \frac{n \pi y}{b} \right] = \frac{16p_0}{D\pi^2} \sum_{m=1,3,5}^{\infty} \sum_{n=1,3,5}^{\infty} \frac{1}{mn} \sin \frac{m \pi x}{a} \sin \frac{n \pi y}{b}$$

• By identification

- For *m* or *n* even:
$$A_{mn} = 0$$

- For *m* & *n* odd:
$$A_{mn} = \frac{16p_0}{D\pi^6 mn} \frac{1}{\left[\left(\frac{m^2}{a^2} + \frac{n^2}{b^2}\right)^2 + \frac{\tilde{n}^{11}m^2}{Da^2\pi^2}\right]}$$

With the solution stated as

•
$$u_3 = \sum_{m=1}^{\infty} \sum_{n=1}^{\infty} A_{mn} \sin \frac{m\pi x}{a} \sin \frac{n\pi y}{b}$$

 $\implies u_3 = \sum_{m=1,3,5}^{\infty} \sum_{n=1,3,5}^{\infty} \frac{16p_0}{D\pi^6 mn \left[\left(\frac{m^2}{a^2} + \frac{n^2}{b^2} \right)^2 + \frac{\tilde{n}^{11}m^2}{Da^2\pi^2} \right]} \sin \frac{m\pi x}{a} \sin \frac{n\pi y}{b}$





- Simply supported plate with tension & constant pressure p_0 (4)
 - Solution

•
$$\boldsymbol{u}_3 = \sum_{m=1,3,5}^{\infty} \sum_{n=1,3,5}^{\infty} \frac{16p_0}{D\pi^6 mn \left[\left(\frac{m^2}{a^2} + \frac{n^2}{b^2} \right)^2 + \frac{\tilde{n}^{11}m^2}{Da^2\pi^2} \right]} \sin \frac{m\pi x}{a} \sin \frac{n\pi y}{b}$$

For a square plate *a=b*

$$\boldsymbol{u}_{3}\left(\frac{a}{2}, \frac{a}{2}\right) = \frac{p_{0}a^{4}}{D} \sum_{m=1, 3, 5}^{\infty} \sum_{n=1, 3, 5}^{\infty} \frac{16}{\pi^{6}mn\left[\left(m^{2} + n^{2}\right)^{2} + \frac{\tilde{n}^{11}m^{2}a^{2}}{D\pi^{2}}\right]} \sin\frac{m\pi}{2} \sin\frac{n\pi}{2}$$



Tension increases the bending stiffness



- Simply supported plate with tension & constant pressure p_0 (5)
 - For a square plate a=b (2)
 - Central displacement:

$$\boldsymbol{u}_{3}\left(\frac{a}{2}, \frac{a}{2}\right) = \frac{p_{0}a^{4}}{D} \sum_{m=1, 3, 5}^{\infty} \sum_{n=1, 3, 5}^{\infty} \frac{16}{\pi^{6}mn\left[\left(m^{2} + n^{2}\right)^{2} + \frac{\tilde{n}^{11}m^{2}a^{2}}{D\pi^{2}}\right]} \sin\frac{m\pi}{2} \sin\frac{n\pi}{2}$$

• If we increase the compression range



- Simply supported plate with tension & arbitrary pressure *p*
 - For constant pressure we found

•
$$\sum_{m=1}^{\infty} \sum_{n=1}^{\infty} A_{mn} \left[\left(\frac{m^4 \pi^4}{a^4} + 2 \frac{m^2 n^2 \pi^4}{a^2 b^2} + \frac{n^4 \pi^4}{b^4} + \frac{\tilde{n}^{11} m^2 \pi^2}{D a^2} \right) \sin \frac{m \pi x}{a} \sin \frac{n \pi y}{b} \right] = \frac{16p_0}{D \pi^2} \sum_{m=1,3,5}^{\infty} \sum_{n=1,3,5}^{\infty} \frac{1}{mn} \sin \frac{m \pi x}{a} \sin \frac{n \pi y}{b}$$

- For an arbitrary pressure we cannot particularize the a_{mn}

•
$$p(x, y) = \sum_{m=1}^{\infty} \sum_{n=1}^{\infty} a_{mn} \sin \frac{m\pi x}{a} \sin \frac{n\pi y}{b}$$

with $a_{mn} = \frac{4}{ab} \int_{0}^{a} \int_{0}^{b} p(x, y) \sin \frac{m\pi x}{a} \sin \frac{n\pi x}{b} dx dy$

General solution

• From
$$u_3 = \sum_{m=1}^{\infty} \sum_{n=1}^{\infty} A_{mn} \sin \frac{m\pi x}{a} \sin \frac{n\pi y}{b}$$

• Equation $D(\boldsymbol{u}_{3,1111} + 2\boldsymbol{u}_{3,1122} + \boldsymbol{u}_{3,2222}) - \tilde{n}^{11}\boldsymbol{u}_{3,11} = p$ leads to

$$\sum_{m=1}^{\infty} \sum_{n=1}^{\infty} A_{mn} \left[\left(\frac{m^4 \pi^4}{a^4} + 2\frac{m^2 n^2 \pi^4}{a^2 b^2} + \frac{n^4 \pi^4}{b^4} + \frac{\tilde{n}^{11} m^2 \pi^2}{D a^2} \right) \sin \frac{m \pi x}{a} \sin \frac{n \pi y}{b} \right] = \frac{1}{D} \sum_{m=1}^{\infty} \sum_{n=1}^{\infty} a_{mn} \sin \frac{m \pi x}{a} \sin \frac{n \pi y}{b}$$

$$\implies A_{mn} = \frac{a_{mn}}{\pi^4 D \left[\left(\frac{m^2}{a^2} + \frac{n^2}{b^2} \right)^2 + \frac{\tilde{n}^{11}m^2}{\pi^2 D a^2} \right]}$$

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Small initial curvature

- Plate with initial curvature
 - This corresponds to
 - An initial mapping along E_3 : φ_{03}
 - Deflection u₃ is measured from this mapping
 - Final mapping along E_3 : $\varphi_3 = \varphi_{03} + u_3$
 - Assumption

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• Initial mapping φ_{03} is small compared to the shell thickness

momentum is still computed

from $\boldsymbol{u}_{3,\alpha\beta}$

$$\implies \tilde{m}^{\alpha\beta} = -\mathcal{H}_m^{\alpha\beta\gamma\delta} \boldsymbol{u}_{3,\gamma\delta}$$

- There is a tension resultant along E_3
 - Previous analysis

$$\left(\mathcal{H}_{m}^{\alpha\beta\gamma\delta}\boldsymbol{u}_{3,\gamma\delta}-\tilde{n}^{\alpha\beta}\boldsymbol{u}_{3}\right)_{,\alpha\beta}=p$$

• But here this effect is proportional to $\varphi_{3,\alpha\beta}$

$$\implies \left(\mathcal{H}_{m}^{\alpha\beta\gamma\delta}\boldsymbol{u}_{3,\gamma\delta}-\tilde{n}^{\alpha\beta}\boldsymbol{u}_{3}-\tilde{n}^{\alpha\beta}\boldsymbol{\varphi}_{03}\right)_{,\alpha\beta}=p$$







- Example: simply supported plate with initial curvature
 - **BCs** for x = 0, *a* **&** for y = 0, *b*
 - $u_3 = 0$ $u_3 = 0$
 - $u_{3,11} = 0$ $u_{3,22} = 0$ (no torque)
 - Constant tension along Ox
 - Equations
 - $\left(\mathcal{H}_{m}^{\alpha\beta\gamma\delta}\boldsymbol{u}_{3,\gamma\delta}-\tilde{n}^{\alpha\beta}\boldsymbol{u}_{3}-\tilde{n}^{\alpha\beta}\boldsymbol{\varphi}_{03}\right)_{,\alpha\beta}=p$
 - Can be developed using

$$- \tilde{m}^{\alpha\beta} = -\mathcal{H}^{\alpha\beta\gamma\delta}_{m} \boldsymbol{u}_{3,\gamma\delta} = -D \left[\nu \delta^{\alpha\beta} \left(\boldsymbol{u}_{3,11} + \boldsymbol{u}_{3,22} \right) + (1-\nu) \, \boldsymbol{u}_{3,\alpha\beta} \right]$$

with $D = \frac{h_0^3 E}{12 \left(1 - \nu^2 \right)}$
 $- \left(\tilde{n}^{\alpha\beta} \boldsymbol{u}_3 \right)_{,\alpha\beta} = \tilde{n}^{11} \boldsymbol{u}_{3,11}$ & $\left(\tilde{n}^{\alpha\beta} \varphi_{03} \right)_{,\alpha\beta} = \tilde{n}^{11} \varphi_{03,11}$

• Final equation

$$- D \left(\boldsymbol{u}_{3,1111} + 2\boldsymbol{u}_{3,1122} + \boldsymbol{u}_{3,2222} \right) - \tilde{n}^{11} \boldsymbol{u}_{3,11} = p + \tilde{n}^{11} \boldsymbol{\varphi}_{03,11}$$

- Solution for
 - p(x,y) = 0?

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 E_3

 φ_{03}

E

 $\tilde{n}^{\hat{1}\hat{1}}$

 E_2

Small initial curvature

- Simply supported plate with initial curvature
 - We can expend the initial mapping in a Fourier series

$$\varphi_{03}(x, y) = \sum_{m=1}^{\infty} \sum_{n=1}^{\infty} b_{mn} \sin \frac{m\pi x}{a} \sin \frac{n\pi y}{b}$$

$$\stackrel{\sim}{\underset{n=1}{\tilde{n}}} \sum_{n=1}^{\infty} b_{mn} \sin \frac{m\pi x}{a} \sin \frac{n\pi y}{b}$$

$$\stackrel{\sim}{\underset{m=1}{\tilde{n}}} \sum_{n=1}^{\infty} \frac{m^2 \pi^2 \tilde{n}^{11}}{a^2} b_{mn} \sin \frac{m\pi x}{a} \sin \frac{n\pi y}{b}$$

- Considering BCs: for x = 0, a & for y = 0, b

$$\begin{cases} \boldsymbol{u}_3 = 0 \\ \boldsymbol{u}_{3,11} = 0 \end{cases} \begin{cases} \boldsymbol{u}_3 = 0 \\ \boldsymbol{u}_{3,22} = 0 \end{cases}$$

The solution can take the form

•
$$\boldsymbol{u}_3 = \sum_{m=1}^{\infty} \sum_{n=1}^{\infty} A_{mn} \sin \frac{m\pi x}{a} \sin \frac{n\pi y}{b}$$





 E_2

 ϕ_{03}

Small initial curvature

- Simply supported plate with initial curvature (2)
 - As we have

•
$$\tilde{n}^{11}\varphi_{03,11}(x, y) =$$

 $-\sum_{m=1}^{\infty}\sum_{n=1}^{\infty}\frac{m^2\pi^2\tilde{n}^{11}}{a^2}b_{mn}\sin\frac{m\pi x}{a}\sin\frac{n\pi y}{b}$
 $\sum_{m=1}^{\infty}\sum_{n=1}^{\infty}\frac{m\pi x}{a}n\pi y$

•
$$\boldsymbol{u}_3 = \sum_{m=1}^{\infty} \sum_{n=1}^{\infty} A_{mn} \sin \frac{m\pi x}{a} \sin \frac{n\pi y}{b}$$

- The governing equation

$$D\left(\boldsymbol{u}_{3,1111} + 2\boldsymbol{u}_{3,1122} + \boldsymbol{u}_{3,2222}\right) - \tilde{n}^{11}\boldsymbol{u}_{3,11} = p + \tilde{n}^{11}\boldsymbol{\varphi}_{03,11}$$

becomes

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$$\sum_{m=1}^{\infty} \sum_{n=1}^{\infty} A_{mn} \left[\left(\frac{m^4 \pi^4}{a^4} + 2 \frac{m^2 n^2 \pi^4}{a^2 b^2} + \frac{n^4 \pi^4}{b^4} + \frac{\tilde{n}^{11} m^2 \pi^2}{D a^2} \right) \sin \frac{m \pi x}{a} \sin \frac{n \pi y}{b} \right] = -\frac{1}{D} \sum_{m=1}^{\infty} \sum_{m=1}^{\infty} \sum_{n=1}^{\infty} \frac{m^2 \pi^2 \tilde{n}^{11}}{a^2} b_{mn} \sin \frac{m \pi x}{a} \sin \frac{n \pi y}{b}$$
$$\implies A_{mn} = -\frac{b_{mn} m^2 \tilde{n}^{11}}{a^2 \pi^2 D \left[\left(\frac{m^2}{a^2} + \frac{n^2}{b^2} \right)^2 + \frac{\tilde{n}^{11} m^2}{\pi^2 D a^2} \right]}$$

• Due to the initial curvature, a tension produces a z-deflection





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 E_2

 φ_{03}

⊮E

Kirchhoff-Love plate summary

Membrane mode

- Completed by appropriate BCs _
 - Dirichlet $oldsymbol{u}_lpha=oldsymbol{ar{u}}_lpha$
 - Neumann $\boldsymbol{n}^{\alpha}_{\beta}\nu_{\alpha} = \mathcal{H}^{\alpha\beta\gamma\delta}_{n} \frac{\boldsymbol{u}_{\gamma,\delta\alpha} + \boldsymbol{u}_{\delta,\gamma\alpha}}{2} \nu_{\alpha} = \bar{\boldsymbol{n}}_{\beta}$





 $\partial_N \mathcal{A}$

 $\frac{n}{n}$

F

• Bending mode

$$- \text{ On } \mathcal{A}: \left(\mathcal{H}_{m}^{\alpha\beta\gamma\delta}\boldsymbol{u}_{3,\gamma\delta}\right)_{,\alpha\beta} = p$$

$$\cdot \text{ With } \mathcal{H}_{m}^{\alpha\beta\gamma\delta} \neq \underbrace{\frac{h_{0}^{3}E}{12\left(1-\nu^{2}\right)}}_{2} \left[\nu\delta^{\alpha\beta}\delta^{\gamma\delta} + \frac{1-\nu}{2}\left(\delta^{\alpha\gamma}\delta^{\beta\delta} + \delta^{\alpha\delta}\delta^{\beta\gamma}\right)\right]$$

- Completed by appropriate BCs
 - Low order

- On
$$\partial_{N}\mathcal{A}$$
: $-\left(\mathcal{H}_{m}^{\alpha\beta\gamma\delta}\boldsymbol{u}_{3,\gamma\delta}\right)_{,\beta}\nu_{\alpha}=\bar{T}$

- On
$$\partial_D \mathcal{A}$$
: $u_3 = ar{u}_3$

• High order

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– On
$$\partial_{_T} \mathcal{A}$$
: $\Delta t = ar{\Delta t}$

with
$$\Delta t = - u_{3,lpha} E_{lpha}$$

- On
$$\partial_{M}$$
A: $-\left(\mathcal{H}_{m}^{lphaeta\gamma\delta}oldsymbol{u}_{3,\gamma\delta}
ight)
u_{eta}=ar{M}
u_{lpha}$





- Membrane-bending coupling
 - The first order theory is uncoupled
 - For second order theory
 - On \mathcal{A} : $\left(\mathcal{H}_{m}^{\alpha\beta\gamma\delta}\boldsymbol{u}_{3,\gamma\delta}-\tilde{n}^{\alpha\beta}\boldsymbol{u}_{3}\right)_{,\alpha\beta}=p$
 - Tension increases the bending stiffness of the plate
 - In case of small initial curvature ($\kappa >>$)
 - On *A*:

$$\left(\mathcal{H}_{m}^{\alpha\beta\gamma\delta}\boldsymbol{u}_{3,\gamma\delta}-\tilde{n}^{\alpha\beta}\boldsymbol{u}_{3}-\tilde{n}^{\alpha\beta}\boldsymbol{\varphi}_{03}\right)_{,\alpha\beta}=p$$

- Tension induces bending effect
- General theories

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- For not small initial curvature:
 - Linear shells
- To fully account for tension effect
 - Non-Linear shells





Kirchhoff-Love plate summary

- FE implementation
 - High order equation $\left(\mathcal{H}_{m}^{\alpha\beta\gamma\delta}\boldsymbol{u}_{3,\gamma\delta}-\tilde{n}^{\alpha\beta}\boldsymbol{u}_{3}-\tilde{n}^{\alpha\beta}\boldsymbol{\varphi}_{03}\right)_{,\alpha\beta}=p$
 - Requires C^{γ} finite-elements
 - Shape functions too complex
 - Can be enforced weakly
 - Not common, usually Reissner-Mindlin plates are implemented







Shell kinematics

- In the reference frame E_i
 - The shell is described by

$$oldsymbol{\xi} = \xi^I oldsymbol{E}_I \;\; ext{with} \; egin{cases} (\xi^1,\,\xi^2) \in \mathcal{A} \ \xi^3 \in [-rac{h}{2};\,rac{h}{2}] \end{cases}$$

- Initial configuration S_0 mapping
 - Neutral plane $arphi_0\left(\xi^1,\,\xi^2
 ight)$
 - Cross section $oldsymbol{t}_0(\xi^1,\,\xi^2)\,,\,\,\|oldsymbol{t}\|=1$
 - Thin body

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$$oldsymbol{x}_0 = oldsymbol{\Phi}_0\left(\xi^I
ight) = oldsymbol{arphi}_0\left(\xi^lpha
ight) + \xi^3oldsymbol{t}_0(\xi^1,\,\xi^2)$$

- Deformed configuration *S* mapping
 - Thin body $x = \Phi\left(\xi^{I}
 ight) = arphi\left(\xi^{lpha}
 ight) + \xi^{3}t(\xi^{1},\,\xi^{2})$
- Two-point deformation mapping $\chi = \mathbf{\Phi} \circ \mathbf{\Phi}_0^{-1}$







• Shell kinematics (2)

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- Deformation gradient
 - Two-point deformation mapping $\ oldsymbol{\chi} = oldsymbol{\Phi} \circ oldsymbol{\Phi}_0^{-1}$
 - Two-point deformation gradient $\mathbf{F} = \boldsymbol{\nabla} \boldsymbol{\Phi} \circ \left[\boldsymbol{\nabla} \boldsymbol{\Phi}_0 \right]^{-1}$

- Small strain deformation gradient $\boldsymbol{\varepsilon} = \frac{1}{2} \left(\mathbf{F} + \mathbf{F}^T \right) - \mathbf{I}$

- It is more convenient to evaluate these tensors in a convected basis
 - Example g_{0I} basis convected to S_0

– As
$$\mathcal{S}_0$$
 is described by $oldsymbol{x}_0 = oldsymbol{\Phi}_0\left(\xi^I
ight) = oldsymbol{arphi}_0\left(\xi^lpha
ight) + \xi^3oldsymbol{t}_0(\xi^1,\,\xi^2)$

- One has $\, oldsymbol{
abla} \Phi_0 = oldsymbol{g}_{0I} \otimes oldsymbol{E}^I \,$ with the convected basis

$$\left\{egin{array}{l} oldsymbol{g}_{0lpha} = rac{\partial oldsymbol{\Phi}_0}{\partial \xi^lpha} = oldsymbol{arphi}_{0,lpha} + \xi^3 oldsymbol{t}_{0,lpha} \ oldsymbol{g}_{03} = rac{\partial oldsymbol{\Phi}_0}{\partial \xi^3} = oldsymbol{t}_0 \end{array}
ight.$$

– The picture shows the basis for $\xi^3 = 0$

»
$$g_{0\alpha} (\xi^3 = 0) = \varphi_{0,\alpha}$$





- Shell kinematics (3)
 - Convected basis g_{0I} to S_0

$$\left\{egin{array}{l} oldsymbol{g}_{0lpha} = oldsymbol{\Phi}_{0,lpha} = oldsymbol{arphi}_{0,lpha} + \xi^3 oldsymbol{t}_{0,lpha} \ oldsymbol{g}_{03} = oldsymbol{rac{\partial oldsymbol{\Phi}_0}{\partial \xi^3} = oldsymbol{t}_0 \end{array}
ight.$$



- If $t_{0,\alpha} \neq 0$ (there is a curvature)
 - Tension and bending are coupled
 - This leads to locking with a FE integration: stiffness of the FE $\rightarrow \infty$
 - Locking can be solved by



- » Using only one Gauss point (reduced integration) bourglass
 modes (zero energy spurious deformation modes)
- » See next lecture
- Both methods lead to complex computational schemes







- Shell kinematics (4)
 - Convected basis g_{0I} to S_0 (2)

$$\left\{egin{array}{l} oldsymbol{g}_{0lpha} = oldsymbol{\Phi}_{0,lpha} = oldsymbol{arphi}_{0,lpha} + \xi^3 oldsymbol{t}_{0,lpha} \ oldsymbol{g}_{03} = oldsymbol{rac{\partial oldsymbol{\Phi}_0}{\partial \xi^3}} = oldsymbol{t}_0 \end{array}
ight.$$

- The basis is not orthonormal
 - A vector component is still defined as $a_I = \boldsymbol{a} \cdot \boldsymbol{g}_{0I}$

- So can
$$oldsymbol{a}=a_Ioldsymbol{g}_{0I}$$
 be written?

» If so
$$\boldsymbol{a} \cdot \boldsymbol{g}_{0I} = a_{\boldsymbol{A}} \underbrace{\boldsymbol{g}_{0J} \cdot \boldsymbol{g}_{0I}}_{\neq \delta_{IJ}} \neq a_{I}$$
 which is not consistent







• Shell kinematics (5)

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- Convected basis g_{0I} to S_0 (3)

$$\left\{egin{array}{l} oldsymbol{g}_{0lpha} = oldsymbol{\Phi}_{0,lpha} = oldsymbol{arphi}_{0,lpha} + \xi^3 oldsymbol{t}_{0,lpha} \ oldsymbol{g}_{03} = oldsymbol{rac{\partial oldsymbol{\Phi}_0}{\partial \xi^3}} = oldsymbol{t}_0 \end{array}
ight.$$

- The basis is not orthonormal (2)
 - A conjugate basis g_0^{I} has to be defined
 - » Such that $oldsymbol{g}_{0I} \cdot oldsymbol{g}_0^J = \delta_{IJ}$
 - » So vector components are defined by

$$a_I = \boldsymbol{a} \cdot \boldsymbol{g}_{0I} ~ \boldsymbol{\&} ~ a^I = \boldsymbol{a} \cdot \boldsymbol{g}_0^I$$

» The vector can be represented by $a = a^{J}g_{0J} = a_{I}g_{0}^{I}$ $as \begin{cases} a \cdot g_{0I} = a_{J}g_{0}^{J} \cdot g_{0I} = a_{I} \\ a \cdot g_{0}^{I} = a^{J}g_{0J} \cdot g_{0}^{I} = a^{I} \end{cases}$ - For plates $\begin{cases} g_{0\alpha} = \varphi_{0,\alpha} = E_{\alpha} = E^{\alpha} \\ g_{03} = t_{0} = E_{3} = E^{3} \end{cases}$







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